

Research Paper

Load Analysis of Trestle Leg Conveyor Support Structures Buried in Stockpiles

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The paper presents a theory, combined with design equations, to compute the loads on structural elements, in particular those of trestle legs of tubular construction supporting load-out belt conveyors, where the trestle legs are substantially buried in gravity reclaim stockpiles. The stress conditions, both active and passive, that arise during filling and reclaim are examined taking into account the position of the trestle supports with respect to the load out conveyor discharge during filling, and reclaim hoppers during draw-down. The theory examines the various loading conditions due to the stored bulk solids focusing on the determination of the lateral or bending load distribution on the trestle support legs. The compressive, in-plane shear traction loads which may influence the buckling effect in the buried legs are also examined.

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Introduction

In the mining industries the storage of bulk solids in large quantities is often achieved through the use of open stockpiles in which the reclaim is by gravity flow through hoppers, feeders and/or chutes onto reclaim conveyors installed below ground level. In many cases the load-out conveyor supplying the bulk solid

to the stockpile is via a boom conveyor supported by a vertical support structure located outside the peripheral area of the of the stockpile rill zone at ground level. The support structure is not in contact with the stored stockpile material.



Fig. 1: Coal stockpile with trestle legs supporting the conveyor (Pictures: ©The Authors/TUNRA Bulk Solids)

In other cases, particularly in the coal industry, the load-out conveyor is supported on trestle legs which are substantially buried in the stockpile as illustrated in Fig. 1. It is this system that is the subject of this paper. The paper builds upon the work of Roberts et al [1-3] which dealt with buried structural members which are square or rectangular in cross-section. In this current study, the trestle legs are hollow tubes of circular cross section which required the load theory to be appropriately modified. As stated by Roberts [1], the loads acting on the trestle legs are quite complex and variable, being dependent on the following

conditions:

- Variations in the flow properties of the material
- Loading and unloading history and mechanism for loading and unloading
- Slumping and avalanching during periods of heavy rain
- Length of time the bulk material remains undisturbed in the stockpile
- Degree of load support provided by the consolidation of the bulk solids acting around the trestle legs
- Rigidity of the stockpile base - whether any ground settlement has occurred
- Variations in stress fields from active to passive during filling and reclaim
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For these reasons, it is recommended that a conservative approach be taken to the trestle leg load calculations. For the efficient operation of the storage and load-out system, it is important that the stockpile geometry and the positioning of the reclaim hoppers and feeders be such that the tonnage of bulk solids reclaimed by gravity flow is achieved. Procedures for achieving this objective are presented by Roberts et al [4,5].

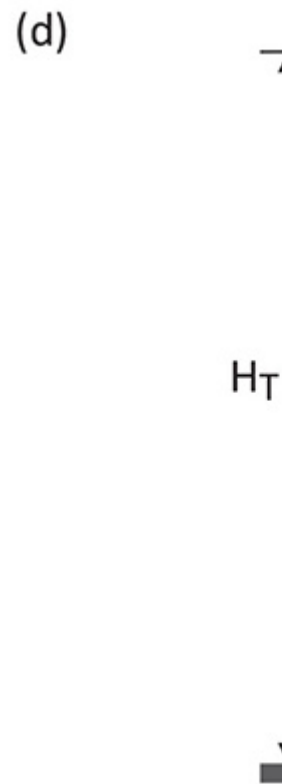
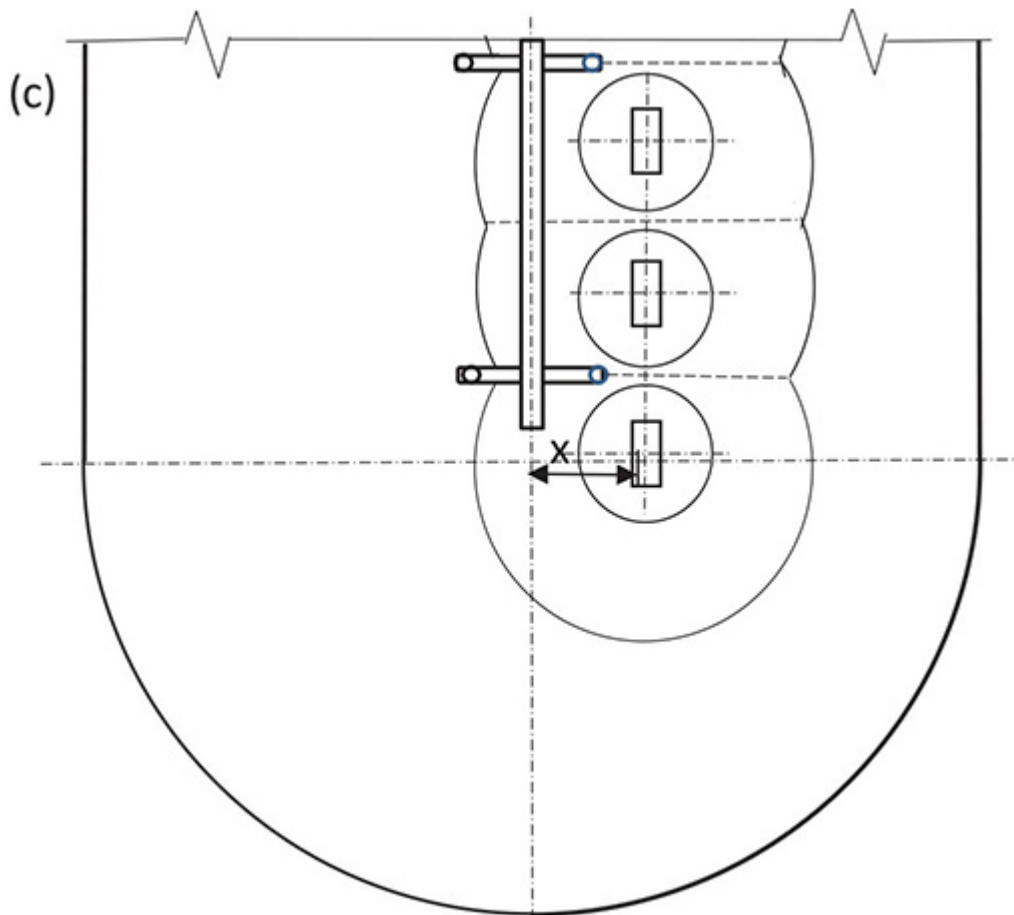
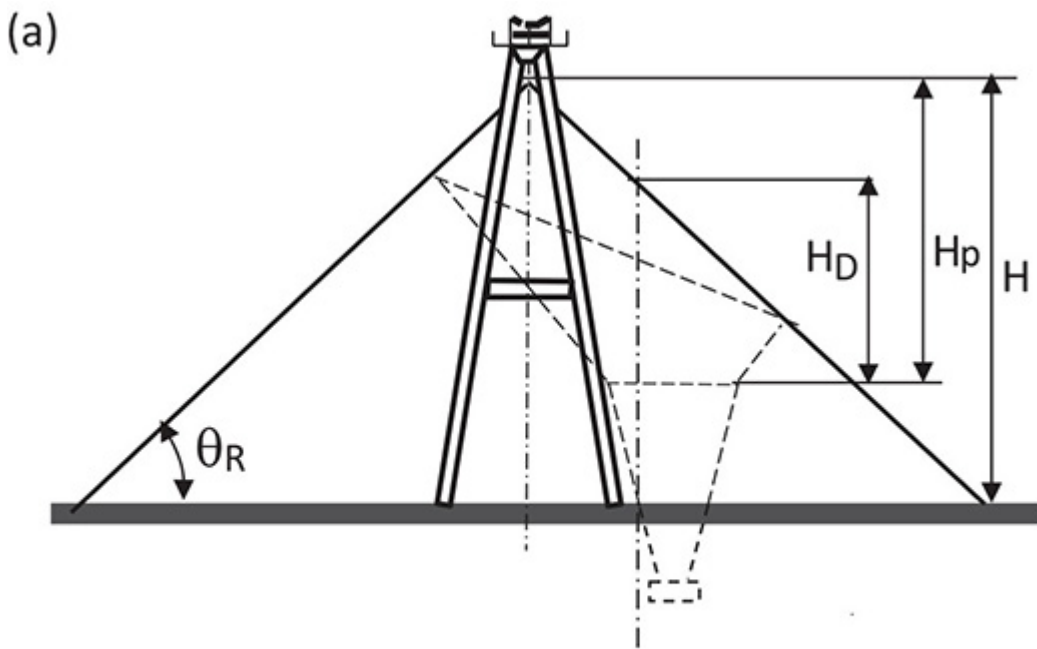


Fig. 2: Schematic arrangement of stockpile showing trestle legs, load-out conveyor and reclaim system

Reference to Fig. 2 shows that the reclaim hoppers, feeders and conveyor are offset an amount defined by "x" from the centre of the stockpile. This is deliberate. Since the reclaim volume is controlled by the draw-down H_D measured from the free surface of the pile, it can be shown that the improvement

in live capacity is possible over that achieved by locating the reclaim centrally. Furthermore, the offset reclaim assists the layout for the central positioning of the loads-out conveyor and its trestle leg supports.

1. Loading Conditions on Trestle Legs

In general, the principal loads are those due to the variation of the normal pressures with depth during the filling and reclaim processes, and the non-uniformity or eccentricity of these pressures acting around the periphery of the trestle legs. The combination of these pressure variations gives rise to bending stresses in the support trestle legs. Furthermore, the cumulative, in-plane type compressive loads due to the shear stresses acting downward along the trestle legs can contribute to the buckling loads, thus exacerbating the lateral bending. Referring to Fig. 2, it is assumed that the stockpile is filled to its maximum height so that the trestle leg structures are fully immersed in the coal with the highest loads occurring on the side of the legs nearest the tripper position during filling operation and on the side of the legs furthest from the reclaim openings during discharge. In view of its relevance to the present study, the original theory of Roberts [1] is reviewed.

1.1 Loading Process - Active Stress State

The trestle leg loads for the active case are depicted in Fig. 3. As the bulk solid is discharged from the loadout conveyor, it slides down the sloping surface of the pile as it is formed, coming to rest with the surface inclined at the angle of repose θ_R of the bulk solid. Within the stored mass, as the depth z increases, the bulk solid continues to move by internal shear at a steeper angle. In this case the slope angle measured from the horizontal approaches that of the effective angle of internal friction δ . For this condition the angle β of Fig. 3 is such that $\beta = (90^\circ - \delta)$.

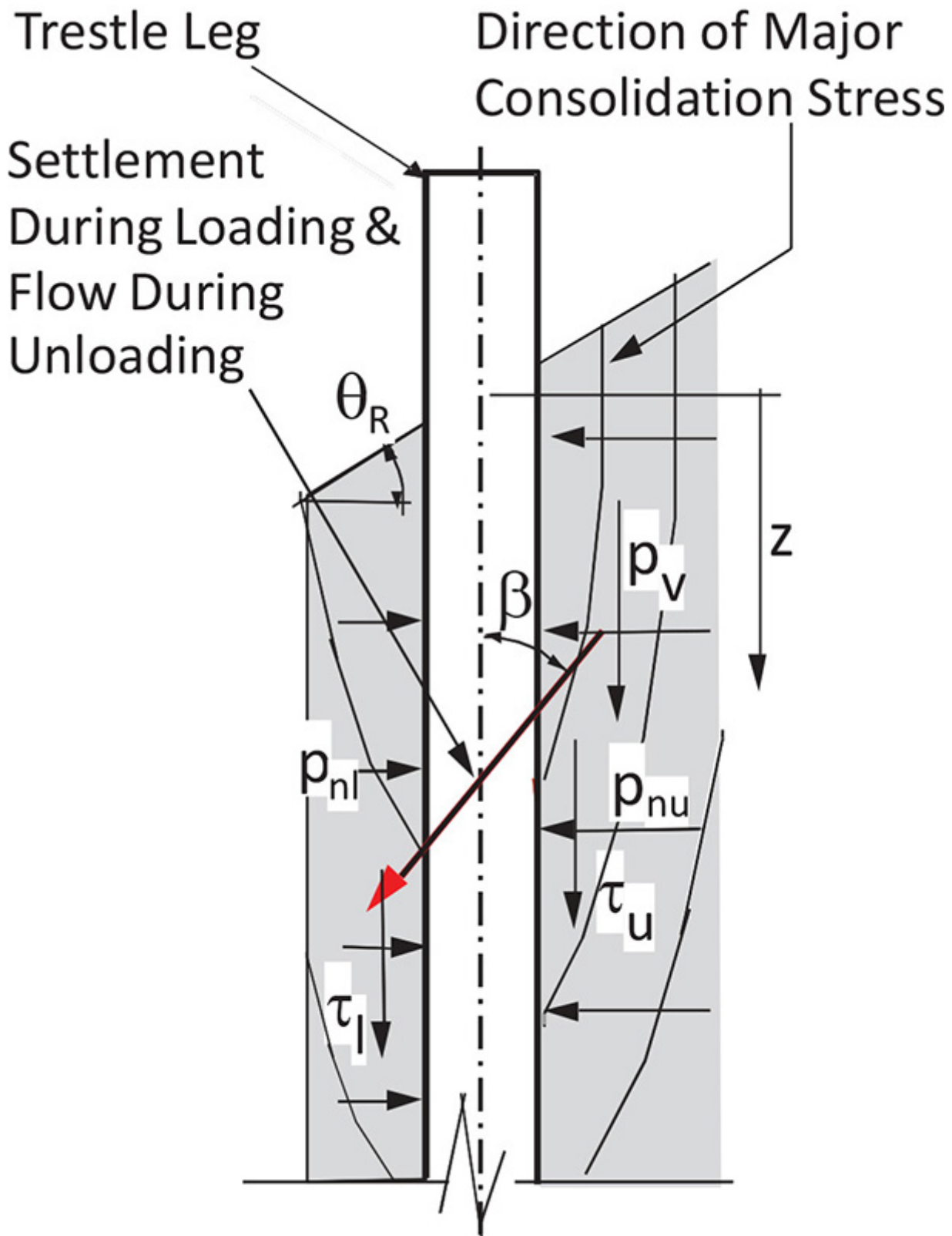


Fig. 3: Column loads - active case

Internal creep due to shear continues for some time after the stockpile is loaded. Depending on the stockpile capacity and the properties of the bulk solid, creep may continue for several hours or even longer periods as the bulk solid approaches, asymptotically, its critical consolidation condition. While this movement is not normally observable being incrementally small as time

increases, nonetheless it does occur. The loading condition is said to be “active” since the major principal consolidating stress acts more in the steeply inclined direction as illustrated in Fig. 3. For cohesive bulk solids, it is also known that the internal creep delays the backfilling of the trestle legs resulting in lower stress levels and, hence, lower support provided by the bulk solid in contact with the trestle legs.

1.2 Load Condition - Passive State of Stress

During storage and reclaim, a “passive” stress state, illustrated in Fig. 4, may be established in the region of the trestle legs. For the storage or filling phase, a passive stress state is possible due to

- settlement during storage,
- lateral deflection due to bending of the trestle legs.
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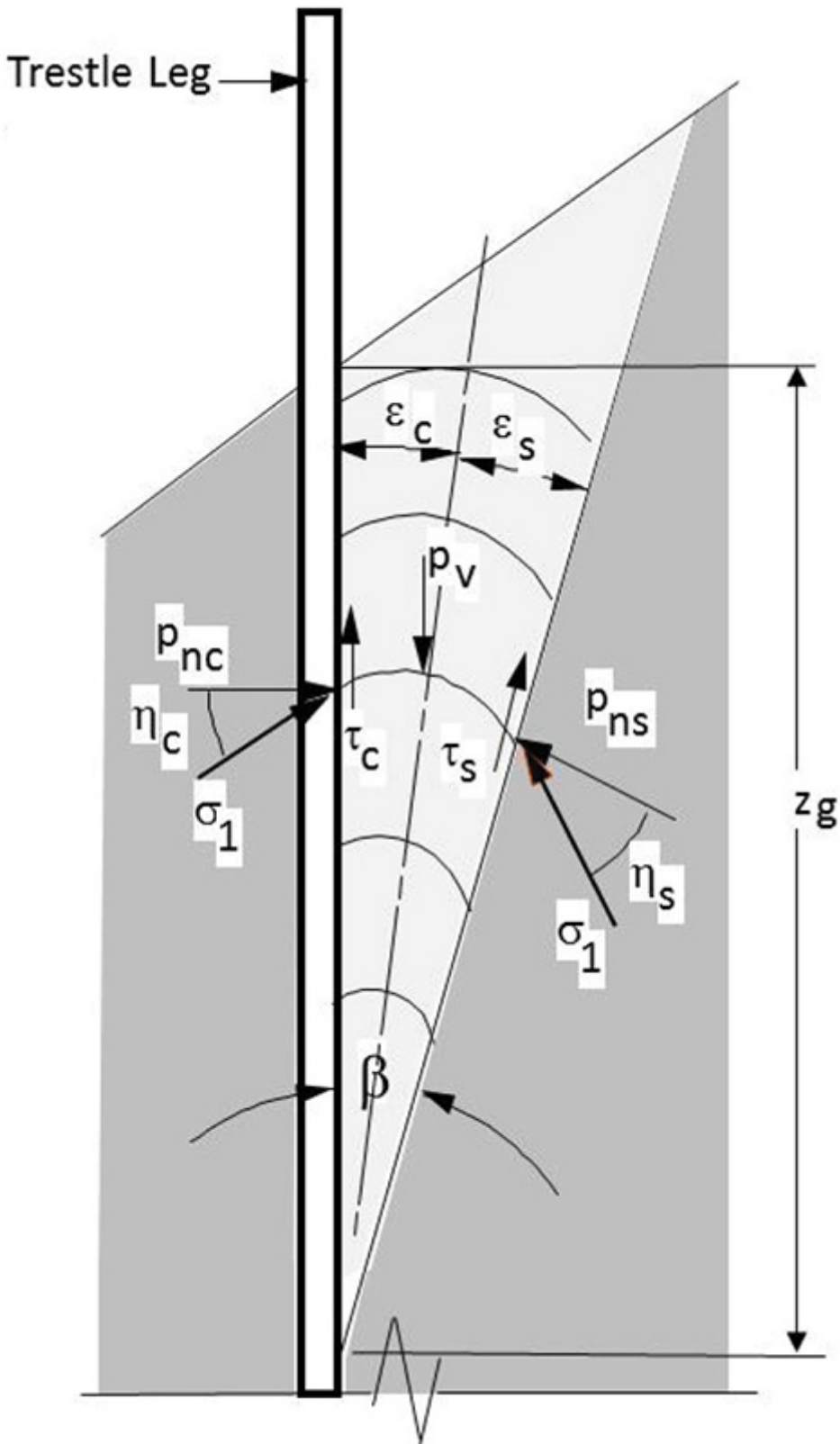


Fig. 4: Column loads - passive case

For the reclaim, gravity flow phase, a passive state of stress occurs with a wedged-shaped flow convergence arising as a result of downward movement during flow. This gives rise to an arching condition in which the major consolidating stresses are more in the lateral direction tangential to the arches. It is assumed that the shear boundary defined by the angle β gives rise to the wedge loading condition as illustrated. The centre line of the wedge is defined by

the angles α_c and α_s which depend on the friction at the column surface and the internal shear surface respectively. It is noted that $\alpha_c + \alpha_s = \beta$.

2. Trestle Leg Load Analysis Model

The model for the trestle leg load analysis for both the active and passive cases is shown in Fig. 5. The dimension z_g defines the depth applicable to the relevant stress field. This dimension, in general, will be different for the active and passive cases. The load analysis model for the pressure and load distribution around the trestle leg is shown in Fig. 6.

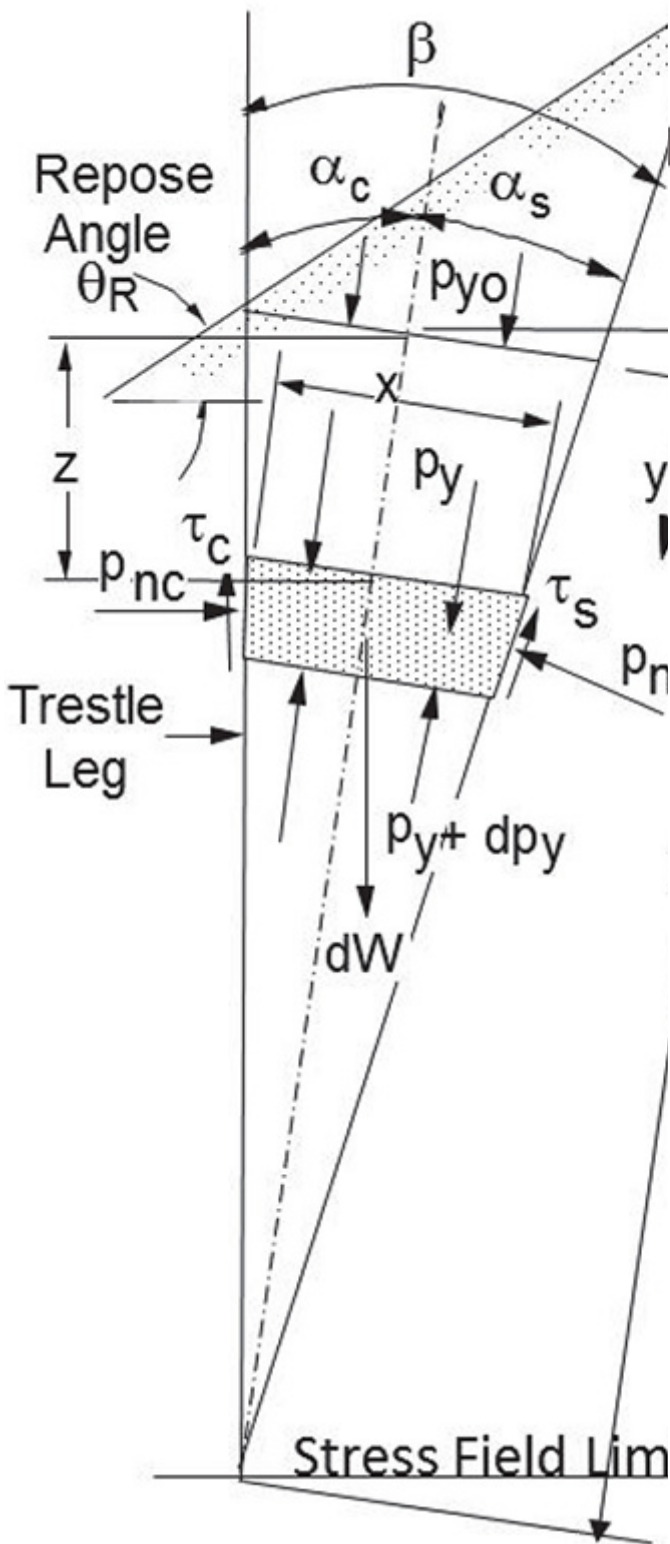


Fig. 5: Load analysis model

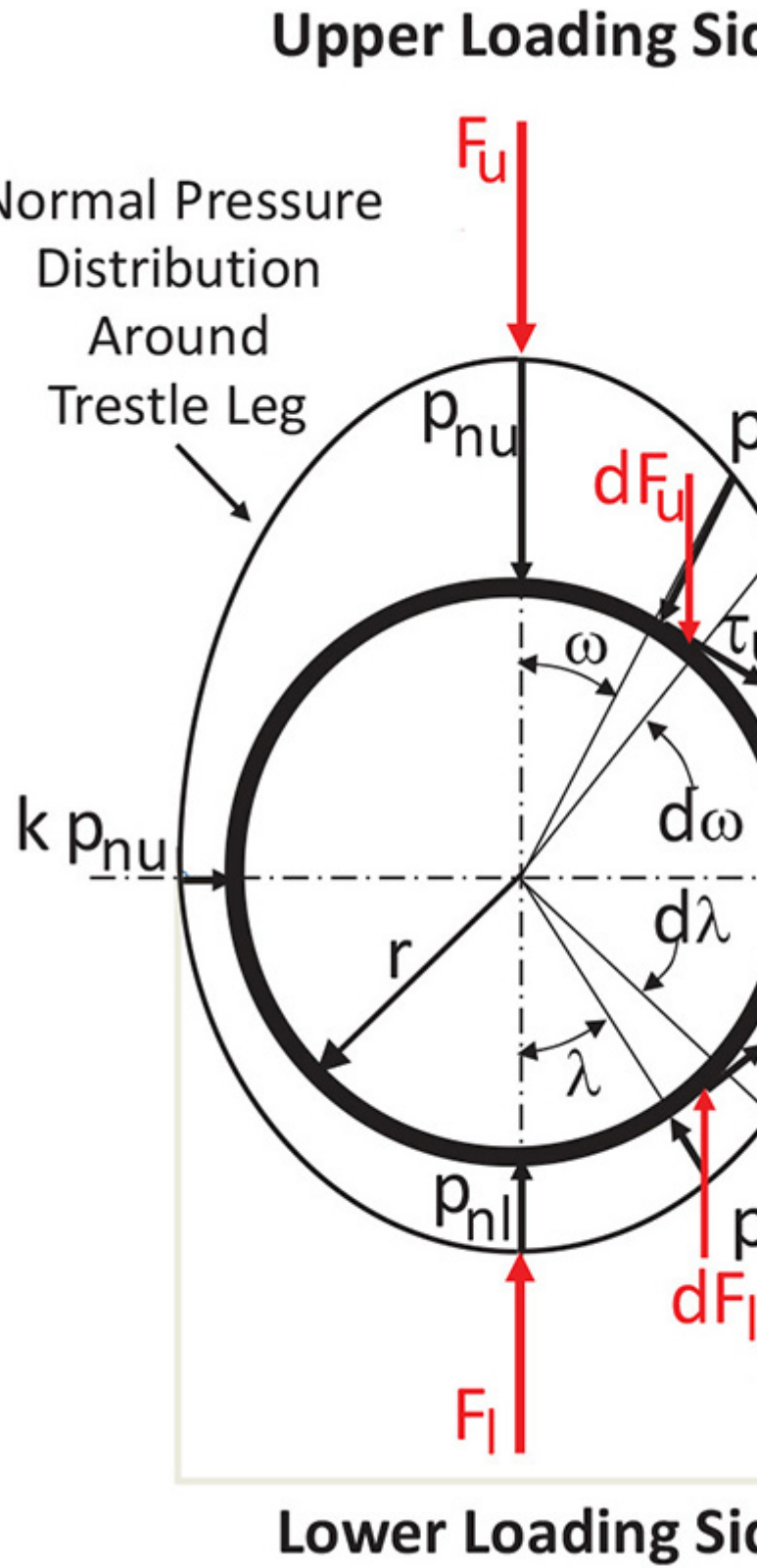


Fig. 6: Pressure distributions and loads

Referring to Fig. 5 and choosing the coordinates "x" and "y", the equilibrium analysis of the elemental slice of thickness dy leads to the following differential equation:

$$\frac{dp_y}{dy} + \frac{j p_y}{y_h - y} = \gamma_y \quad (1)$$

The solution of this equation for the pressure distribution is

$$p_y = \gamma_y \left[\frac{y_{h-y}}{j-1} \right] + \left[\left(p_{y0} - \frac{\gamma_y y_h}{j-1} \right) \left(\frac{y_{h-y}}{y_h} \right)^j \right] \quad (2)$$

with

$$j = \frac{K_c (\tan \alpha_c + \tan \phi_w) + K_s (\tan \alpha_s + \sin \delta)}{\tan \alpha_c + \tan \alpha_s} \quad (3)$$

and

$$K_{ca} = \frac{p_{nc}}{p_y}$$

$$K_s = \frac{p_{ns}}{p_y}$$

$$\gamma_y = \gamma \cos \alpha_c$$

where γ = bulk specific weight, (kN/m³); ϕ_w = wall friction angle for column surface; δ = effective angle of internal friction; p_{y0} = surcharge pressure; $y = z (\cos \alpha_c)^{-1}$

2.1 Active Case

Based on the discussion in Section 1.1, the angle $\beta = (90^\circ - \delta)$. For the active loading condition it is reasonable to assume that $\alpha_c = \alpha_s = \beta/2$ for which the pressure ratios, $K_c = K_{ca}$ and $K_s = K_{sa}$, are given by

$$K_{ca} = (j+1) \left[\frac{\tan \alpha_c}{\tan \alpha_c + \tan \phi_w} \right] \quad (4)$$

and

$$K_{sa} = (j+1) \left[\frac{\tan \alpha_s}{\tan \alpha_s + \sin \delta} \right] \quad (5)$$

The analysis assumes that on the slip plane the coefficient of internal friction corresponds to the maximum shear stress. Hence,

$$\mu_i = \sin \delta = \tan \phi_{wi} \quad (6)$$

where ϕ_{wi} = equivalent internal friction angle. The parameter 'j' in Eqs. (4) and (5) allows for the degree of compressibility of the bulk solid and the rigidity of the stockpile base. In this case, it is assumed that $0 \leq j \leq 0.2$. The maximum limiting case for the pressure p_y occurs when $j = 0$ for which the pressures p_y , p_v , p_{nc} and p_{ns} are 'hydrostatic', increasing linearly with depth as follows:

$$p_y = p_{y0} + \gamma_s y \quad (7)$$

and

$$p_v = p_{v0} + \gamma z \quad (8)$$

as well as

$$p_{nc} = K_c (p_{y0} + \gamma_s y) \quad (9)$$

and

$$p_{ns} = K_s (p_{ys} + \gamma_s y) \quad (10)$$

2.2 Passive Case

In this case the stress field is as shown in Fig. 5 with the angle β of the failure plane defined as $\beta = 45^\circ - \delta/2$. It is assumed that failure on the slip plane occurs at the maximum shear stress. Eq. (2) applies in this case with the pressure ratios $K_c = K_{cp}$ and $K_s = K_{sp}$ which need to be selected in accordance with passive stress field model. Two methods are considered for the determination of these stress ratios, one based on the Australian Standard AS3774 on "Loads in Bulk Solids Containers" [6], and the other a modified version of the AS3774 method. In accepting that the flow in the region of the trestle legs is a complex mixture of axi-symmetric and plane flow, it is reasonable to assume plane flow is the dominant mode of flow.

2.2.1 Method based on AS3774

In this case the relationships for the K values are given by

$$K_{cp} = \frac{1 + \sin \delta \cos 2\eta_c}{1 - \sin \delta \cos 2(\alpha_c + \eta_c)} \quad (11)$$

and

$$K_{sp} = \frac{1 + \sin\delta \cos 2\eta_s}{1 - \sin\delta \cos 2(\alpha_s + \eta_s)} \quad (12)$$

where

$$\eta_c = 0.5 \left[\phi_w + \sin^{-1} \left(\frac{\sin \phi_w}{\sin \delta} \right) \right] \quad (13)$$

and

$$\eta_s = 0.5 \left[\phi_{ws} + \sin^{-1} \left(\frac{\sin \phi_{ws}}{\sin \delta} \right) \right] \quad (14)$$

2.2.2 Modified Method

Experience in hopper design has shown the AS3774 method sometimes underestimates the wall loads in plane flow hoppers. This has led to the modified pressure ratios as follows:

$$K_{cpm} = \frac{2(1 + \sin\delta \cos 2\eta_c)}{1 - \sin\delta (1 + \cos 2(\alpha_c + \eta_c))} \quad (15)$$

$$K_{spm} = \frac{2(1 + \sin\delta \cos 2\eta_s)}{1 - \sin\delta (1 + \cos 2(\alpha_s + \eta_s))} \quad (16)$$

2.3 Support Loads on Rear Lower Face of Columns

As previously discussed, the back-pressure support against lateral deflection of the trestle legs is uncertain and, generally during filling and reclaim, will be of low order, particularly in the case of cohesive bulk solids. The most conservative approach is to assume that the pressure p_{nl} (Figs. 2 and 4) is zero. However, it is suggested that a more reasonable approach is to calculate the back pressure as follows:

$$p_{vl} = \gamma z \quad (17)$$

and

$$p_{nl} = k_{cr} \gamma z \quad (18)$$

It may be shown that the ratio of normal to vertical stress, k_{cr} , is given by

$$k_{cr} = \frac{p_{nl}}{p_{vl}} = \frac{1 + \sin^2 \delta - 2\sqrt{(\sin^2 \delta - \tan^2 \phi_w \cos^2 \delta)}}{4\tan^2 \phi_w + \cos^2 \delta} \quad (19)$$

This equation corresponds to that given in Section 6.2.1.1 of AS3774 [6]. In this paper, for example, the values $\delta = 50^\circ$ and $\phi_w = 30^\circ$ are assumed as being relevant for the coal stored in the stockpile. Applying Eq. (19) gives $k_{cr} = 0.14$. It is noted that AS3774 adopts the lower bound value pressure ratio of 0.35 for the active stress state for the wall load determination in cylindrical bins and silos. When extrapolating this information to the stockpile trestle leg loads, it is contended that the lower value of k_{cr} calculated using Eq. (19) is relevant so as to avoid over-estimating the lateral support by coal on the rear or backfilling face of the trestle legs.

2.4 Pressure Distribution around Trestle Leg and corresponding Loads

Referring to Fig. 5, the pressure p_{nu} is the peak pressure as determined in accordance with Sections 3.1 and 3.2 for the active and passive cases respectively.

2.4.1 Upper or Loading Half of Trestle Leg

At an arbitrary position define by the angle ω , the normal pressure is defined by

$$p_{n\omega} = k_\omega p_{nu} \quad (20)$$

where k_ω is expressed by

$$k_\omega = 1 - (1 - k) \sin \omega \quad (21)$$

such that $0.35 \leq k \leq 1.0$. It is noted that for the lower bound value of $k = 0.35$, $k_\omega = 1$ for $\omega = 0$, and $k = 0.35$ when $k_\omega = \pi/2$. It is assumed there is some build-up of bulk solid on the trestle surface, so that the friction coefficient μ_i defining the shear stress τ is given by Eq. (6). The elemental force per unit length dF_u is given by

$$dF_u = p_{n\omega} r [\cos \omega + \mu_i \sin \omega] d\omega \quad (22)$$

The total force F_u per unit length is given by

$$F_u = 2p_{nu} r \int_0^{\pi/2} [1 - (1 - k) \sin \omega] [\cos \omega + \mu_i \sin \omega] d\omega \quad (23)$$

which becomes

$$F_u = p_{nu} D_e \quad (24)$$

where D_e is the effective trestle leg diameter and is given by

$$D_e = 2rC \quad (25)$$

and

$$C = \left[1 - \frac{1-k}{2} \right] + \mu_j \left[1 - \frac{(1-k)\pi}{4} \right] \quad (26)$$

with r = outside radius of trestle leg. The recommended lower bound value is $k = 0.35$, which, for $d = 50^\circ$, gives $C = 1.05$ and $D_e = 1.05 \cdot D$, where $D = 2 \cdot r$ = actual trestle leg outside diameter. The value $k = 1.0$ corresponds to the pressure p_{nu} being constant around the upper loading half of the trestle leg. For this maximum or upper bound case, $C = 1.7$, for which $D_e = 1.7 \cdot D$.

2.4.2 Backfilling Half of Trestle Leg

This is determined in a similar way to the above. In this case, when $\lambda = 0$, $k_\omega = k_n$ as defined by Eq. (19).

2.5 Effective Trestle Leg Diameter for Load Calculations

It is noted that AS3774 recommends that the load calculations for buried structural elements be based on the projected area corresponding to $D_e = 2 \cdot D$. The Standard seems to be referring more to horizontal or lateral support tie beams in hoppers and silos rather than vertical or steeply inclined support legs buried in stockpiles. It then becomes a value judgement as to whether the factor of 2 should be applied in the case of the trestle leg loads. It needs to be noted that, over a period of time, progressive adhesion and build up may occur around the trestle legs, thus increasing the effective diameter. This is more likely in the case of bulk materials exhibiting a high level of cohesion. If this is a distinct likelihood, then the conservative approach is to select $D_e = 2 \cdot D$. On the other hand, the analysis presented in Section 2.4.1 above shows that the lower and upper bound values to be $C = 1.05$ and $C = 1.7$, respectively. Weighing up the various factors, the recommended lower limit is $C = 1.2$.

2.6 Trestle Leg Compressive Loads

The vertical shear stresses acting along the column faces give rise to compressive loads which increases down the column. At an arbitrary depth location, z , corresponding to coordinate y , the vertical shear stress on the column face is given by

$$\tau_z = \mu_i p_y K \quad (27)$$

where $K = K_c$ for the active case and $K = K_{cp}$ for the passive case. At depth z , the cumulative compressive force per metre of trestle leg circumference is given by

$$F_C = \int_0^z \tau_z dz \quad (\text{kN/m}) \quad (28)$$

Noting that the vertical pressure p_y is given by Eq. (2), then substituting for p_y in (26), for τ_z in Eq. (27) then integrating gives the following expression for the compressive force per unit circumference,

$$F_C = \frac{C_1}{2} [z_h^2 - (z_h - z)^2] + \frac{C_2}{j+1} [z_h^{j+1} - (z_h - z)^{j+1}] + F_{z_0} \quad (29)$$

where

$$C_1 = \frac{\mu K_c \gamma}{j-1} \quad (30)$$

and

$$C_2 = \frac{\mu K_c}{z_h^j} \left[p_{z_0} - \frac{\gamma z_h}{j-1} \right] \quad (31)$$

with $\mu_1 = \tan \phi_w$ which allows for some build-up of bulk solid on the trestle leg surfaces, and F_{z_0} = surcharge load on each leg due to the weight of the conveyor, its load and the support structure. While the normal stress and corresponding shear stress will vary around the leg circumference as illustrated in Fig. 6, it is recommended that the conservative approach for the compressive loads be adopted by assuming the normal and stresses are the maximum values uniformly distributed around the periphery of the leg.

3. Trestle Leg Design Example

A particular case of the load-out coal stockpile and reclaim system of Fig. 2 is now considered. The relevant details are as follows

- Stockpile height fully loaded $H_s = 32$ m
- Trestle leg height $H_T = 34$ m
- Trestle leg slope angle $\varepsilon = 10^\circ$
- Leg spacing $B_T = 12$ m
- Leg diameter $D = 1.2$ m
- Active stress zone $z_{ga} = 31$ m
- Passive stress zone $z_{gp} = 22.4$ m
- Reclaim hopper offset $X = 7$ m
- Angle of repose $\theta_R = 38^\circ$
- Bulk density of coal $\rho = 1.0$ t/m³
- Trestle friction angle $\varphi_s = 30^\circ$
- Static internal friction $\varphi_t = 40^\circ$
- Effective internal friction angle $\delta = 50^\circ$

• Relevant results are presented in Figs. 7 to 10. Fig. 7 shows the normal pressures for the active and passive states for the upper loading face and rear and backfilling face. The corresponding net normal pressures acting on the loading faces of the trestle legs are shown in Fig. 8. These pressures are used to calculate the corresponding loads in accordance with Sections 2.4 and 2.5. Fig. 9 shows the circumferential normal pressure distributions around the trestle legs at critical levels for the passive case at the depth 8.3 m and for the active case at the depth of 27 m. For the active case, the parameter $j = 0.2$ is assumed. Fig. 10 shows the compressive loads per metre of circumference acting on the trestle legs.

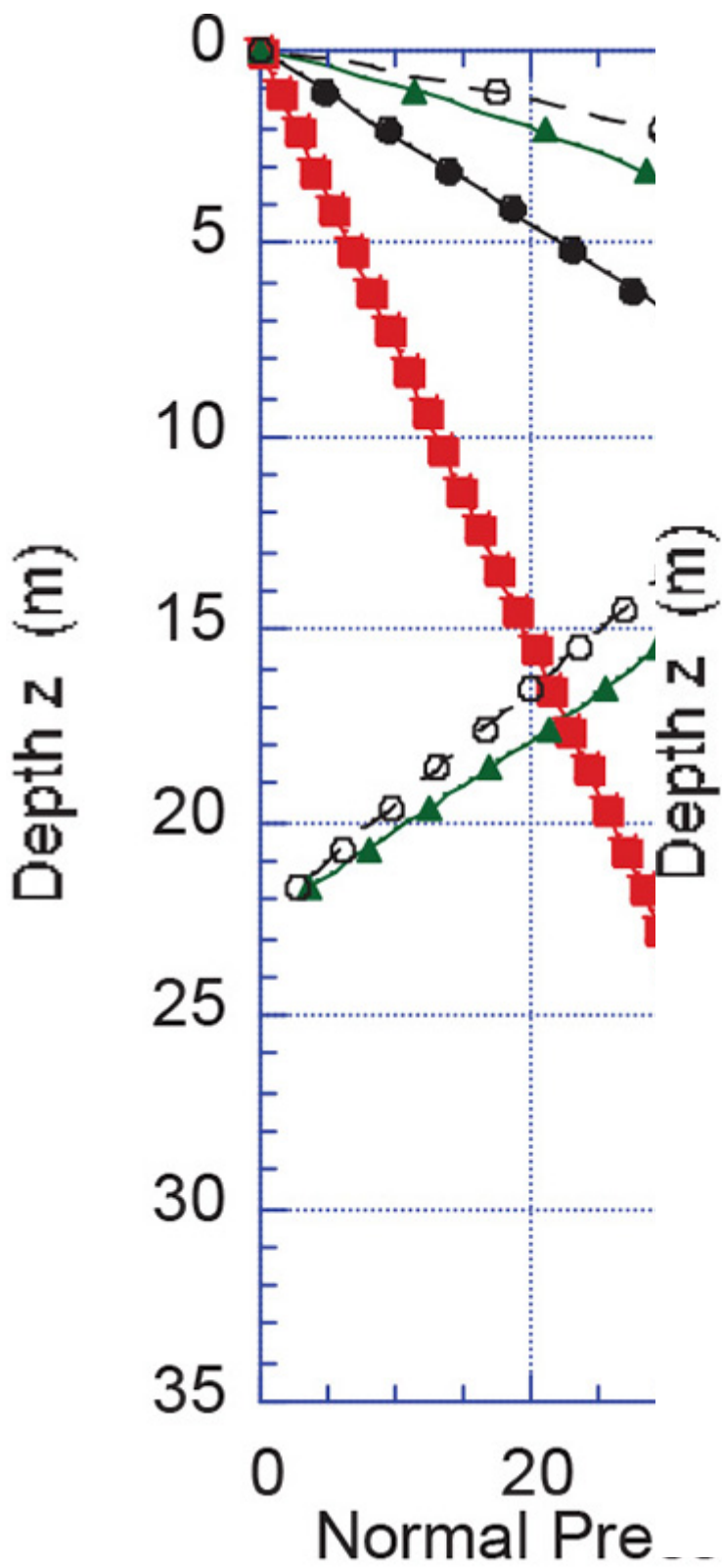


Fig. 7: Normal pressures on trestle legs

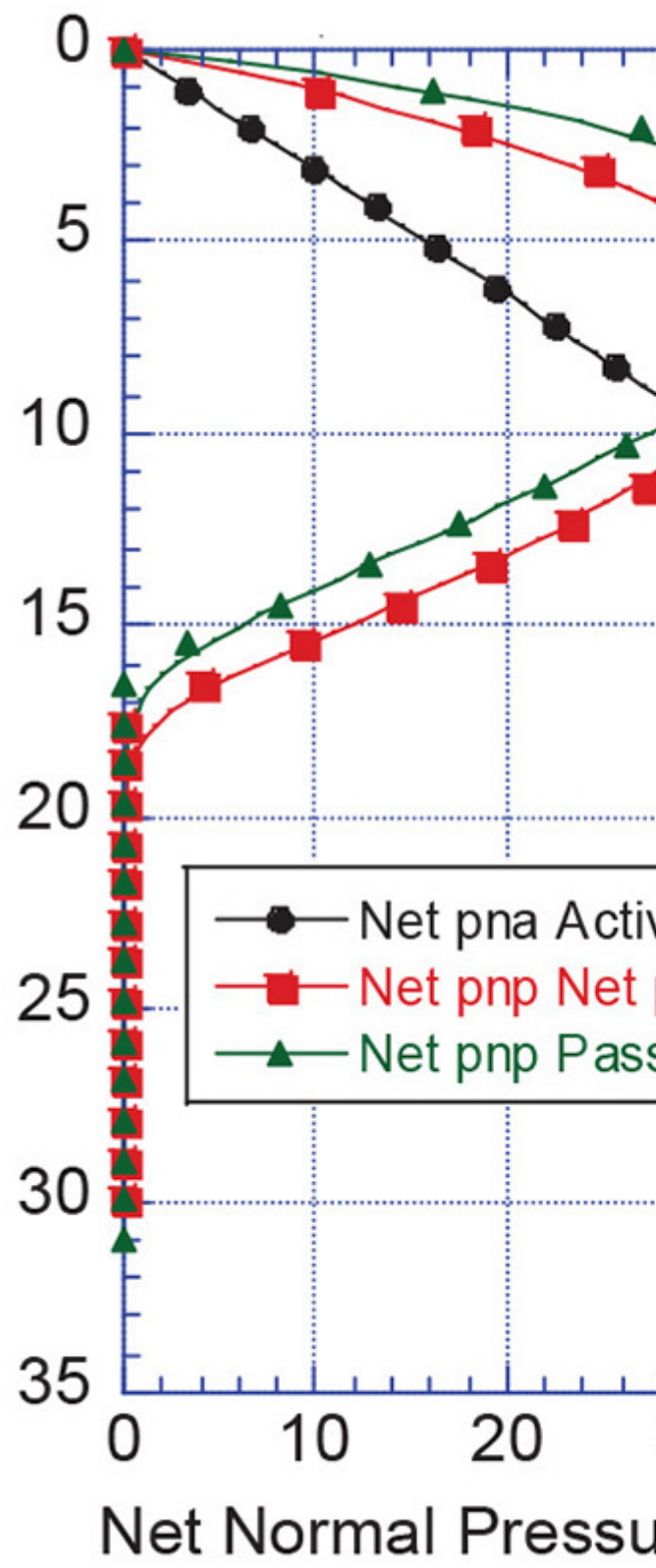


Fig. 8: Net normal pressures on trestle legs

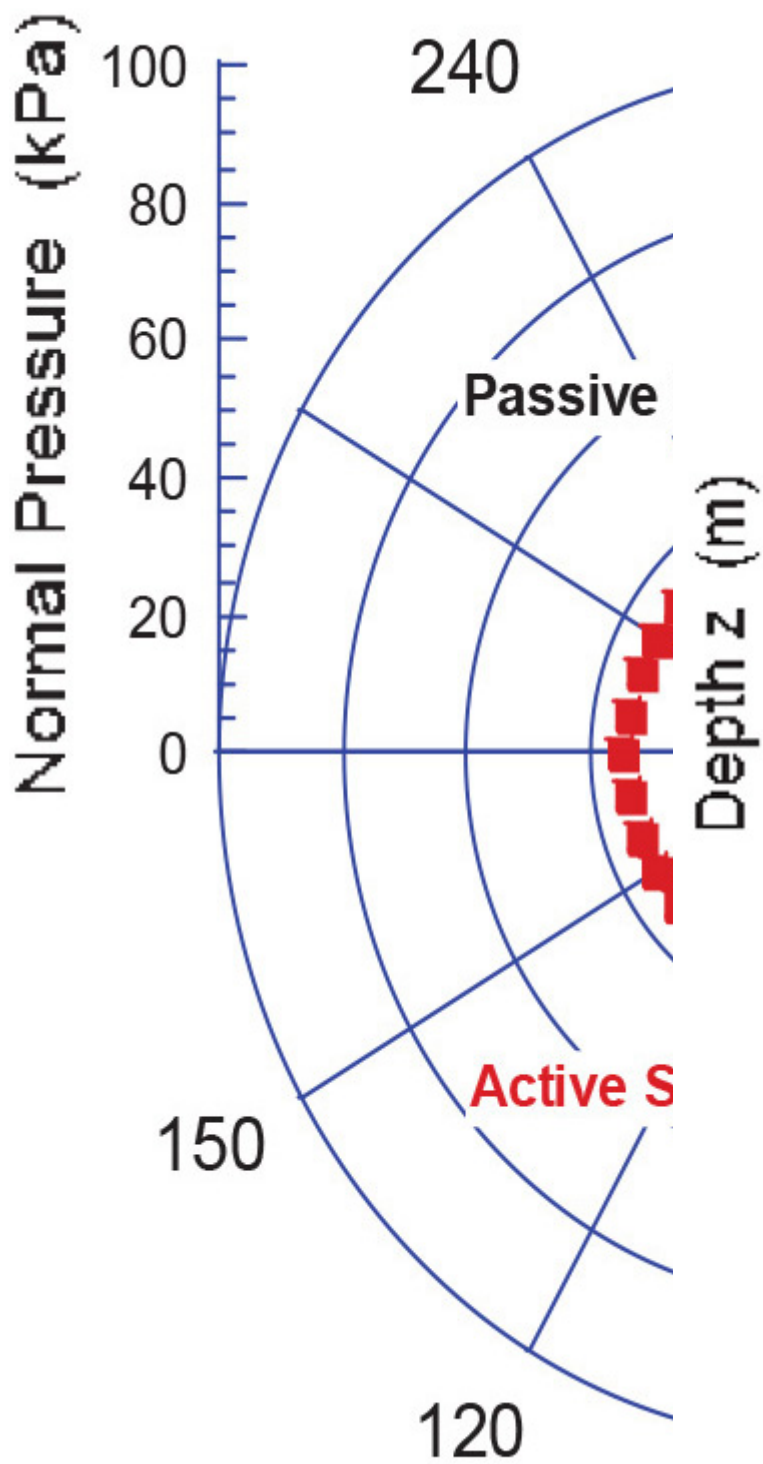


Fig. 9: Circumferential pressures distribution

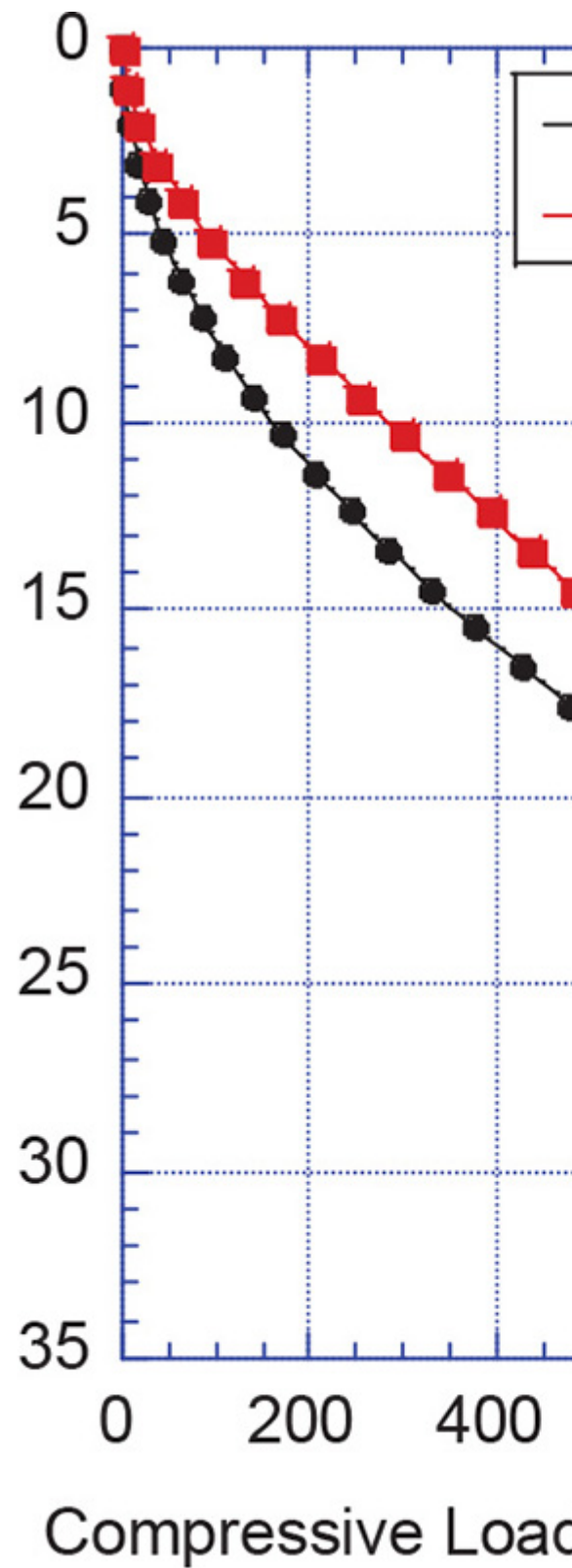


Fig. 10: Compressive loads per meter circumference

4. Concluding Remarks

The loading conditions experienced in conveyor support trestles buried in coal stockpiles are both complex and variable. The loads are a combination of lateral

loads due to filling and reclaim and in-plane compressive loads due to the draw-down forces. The magnitude and distribution of the loads over the trestle leg faces depend very much on the properties of the coal and the manner in which the stockpile is formed and reclaimed. The loads result from an active stress state during filling and from a passive stress state during reclaim. The degree of lateral support due to the backfilling is quite uncertain and should be ignored if there is any doubt as to the possible level of support that is provided. It is important that the compressive forces in the buried trestle legs due to the cumulated shear or traction forces acting downward around the leg surfaces be taken into consideration. These forces contribute to the compressive buckling loads in the legs by adding to the compressive structural loads that exist in supporting the load-out conveyor. Due to the uncertainty of the loading conditions, it is important that a conservative approach be adopted in the design. It is also important to note that any loads due to mobile equipment working on the stockpile, as well as dynamic effects, slumping and possible avalanching due to high increases in moisture are not included in the loads determined herein. This paper is part of ongoing research into the subject of loads on buried structures. Follow up research is currently being undertaken through the integration of DEM and FEA analysis of the trestle leg loads. The results of this follow-up work will be correlated with the predictions of the continuum mechanics approach described in this paper with a view to establishing a reliable design methodology for predicting the loads in trestle leg structures.

Acknowledgement:



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